

ultralamTM



LVL 2.0E/2900Fb

Allied Structural Materials, LLC

Allied Structural Materials (ASM) is the exclusive national manufacturer's representative for Ultralam LVL in the USA. Having decades of experience in marketing engineered wood products, our team knows what is needed to fulfill your LVL supply requirements.



Currently, we are stocking inventory in the ports of Baltimore, Tampa, Cleveland and Houston. We have the capability to provide customized container deliveries from the mill to any designated location. Cut-to-length bundles are available in one foot increments in any length from 8' to 38' at prices well below the stock 48' lengths at the ports.

Our mission is to earn your business every day and make it feel like your LVL manufacturer is next door to each one of your locations.

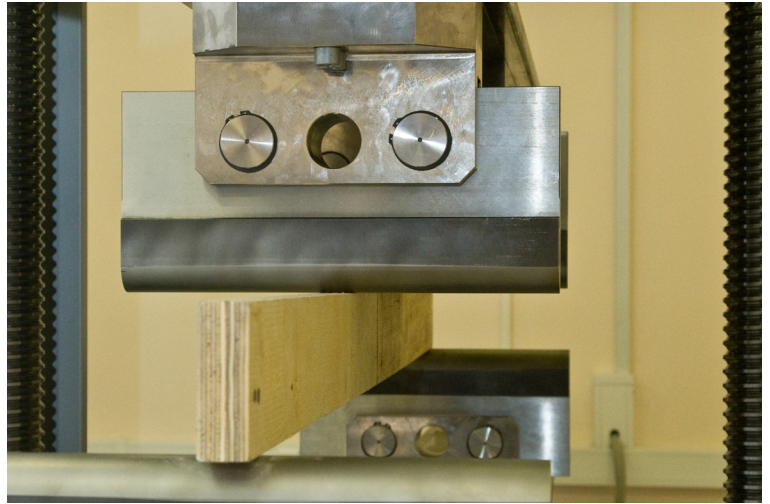


Our Warranty

Ultralam certifications, assuring the ultimate in reliability, are unmatched in the industry. However, for added customer confidence, ASM carries an occurrence-based insurance policy with a \$10 million umbrella, in addition to Modern Lumber Technology, Ltd's manufacturer's warranty.

Ultralam LVL—Reliability

Independently certified by agencies in the United States, Australia, Europe, Japan and the United Kingdom, Ultralam LVL is an extremely reliable product. Daily in house monitoring of the modulus of elasticity and fiber bending, along with monthly onsite inspections by each certifying agency, provides the assurance that every member will perform to its published strength properties. These strength properties, some of which are the highest in the industry, are developed through extensive ASTM testing.



Ultralam LVL, having the highest compression perpendicular-to-grain value at 900 psi, results in shorter bearing lengths. Multiple member connections also provide higher load values for single-side loaded conditions. This reduces the number of nails or size and number of bolts required versus other LVL with lower values.

Uniformity and reliability are obtained using the latest manufacturing equipment, but it starts with the tree source. Ultralam LVL comes from old growth forests that are leased for the next forty years. The

trees are harvested under Forest Stewardship Council (FSC) supervision and the veneers are produced at the plant, thus controlling the source, cost and quality of the wood fiber used in every beam. All of the veneers are scarfed on the ends to assure uniformity in billet thickness. The 60-meter-long continuous press manufactured by Dieffenbacher, the world leader in LVL manufacturing equipment, provides the ultimate in manufacturing control.

In summary, you can count on Ultralam LVL to be a reliable and cost efficient part of your structure for the expected life of each building.



An Elite LVL Manufacturing Facility

The Taleon-Terra plant was specifically designed for manufacturing Ultralam LVL. Utilizing state-of-the-art equipment from the United States and Germany, it has an enormous production capacity. Cutting-edge technology implemented includes the world's longest continuous press (60 meters). With this press, the length of LVL beams can be custom cut to meet the demands of our customers and transport conditions. Integrating this advanced technology of continuous pressing, in conjunction with microwave pre-heating, improves bonding, enhances resin propagation into the wood fibers, and yields new, uniform, high-strength material.



60 Meter Continuous Press

Environmentally conscious and committed to sustaining natural resources, Ultralam production is non-waste. All remains are recovered and used for other products. The logging site has been certified by FSC and meets all of their requirements.

These production protocols and techniques make Ultralam a high-strength laminated veneer lumber that is cost effective,

easy to use, and environmentally friendly for the construction of energy-saving structures.



Production Snap Shot

Modern, high-tech wood processing techniques are utilized in the production of Ultralam LVL. Pine and Spruce are harvested from the logging site and then delivered to the mill where they are sorted by species, quality and size. Logs are debarked, conditioned, slashed and trimmed into 2.65 m peeling blocks. These blocks are scanned by laser beams to ensure optimum block positioning and the highest veneer recovery. A computerized video control and scanner system reveals defects



precluding their presence in veneer sheets. Peeling of the blocks is performed at a very high speed, 18 blocks per minute. Control systems read and calculate moisture content for each sheet before they are dried in a 6-level roller-type thermal oil dryer.

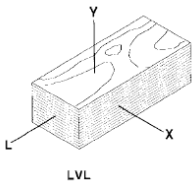
From the dryer outfeed, further veneer testing takes place on the sorting conveyor. Ultrasonic signals are used to measure signal propagation time to determine density. Veneers are separated into four grades. Only A-class veneers are used for structural LVL manufacturing. High water resistance and low emissions class glue is then applied to the veneers via curtain coater before proceeding to the lay-up section.

The 2-level lay-up section loads the veneer, places it on the conveyor and then shuttles it to the press infeed section. Veneer fan is continuously formed and conveyed to the microwave pre-heater incorporated into the hot press. In the continuous press, the LVL billet is conveyed with constant speed through several zones with different temperature and pressure settings where it is densified to the required thickness. Upon exit, billets pass through the blow detector and thickness meter before the sides are hogged to final dimension and cut by a diagonal saw to the required length. After at least 24 hours hold time, billets are cut at the rip saw.

Boards are then conveyed to the packaging line to be stamped and stacked. LVL stacks are cut automatically by length, edges are squared and the boards are wrapped with a special film and banded with metal strap for storage.

Reference Design Values for Ultralam™ LVL (Allowable Stress Design)^{1, 2, 3, 4}

Bending, F _b (psi) (Mpa)	Tension, F _t (psi) (Mpa)	Compression, F _c (psi) (Mpa)		Horizontal Shear, F _v (psi) (Mpa)	Modulus of Elasticity, E (psi) (Mpa)		Modulus of Elasticity for Beam & Column Stability, E _{min} (psi) (Mpa)	Shear Modulus of Elasticity, G (psi) (Mpa)
		Parallel- to-Grain ⁸	Perpendicular- to-Grain ⁹		Beam	True ⁵		
2900 (20.0)	2150 (14.8)	3150 (21.7)	900 (6.2)	320 (2.2)	2.0x10 ⁶ (13790)	1.9x10 ⁶ (13100)	1.0x10 ⁶ (6895)	125,000 (862)

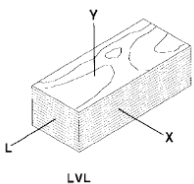


- 1 psi = 0.00689 MPa or 1 MPa = 145 psi.
- The reference design values in this table are applicable for the product used in dry, well-ventilated interior applications, in which the equivalent moisture content of sawn lumber is less than 16%.
- The reference design values in this table are for normal load duration. Loads of longer or shorter duration shall be adjusted in accordance with the applicable code. Duration of load adjustments shall not be applied to F_{cPerp}, E, E_{min} and G.
- Orientation nomenclature for Ultralam™ LVL.
- The Apparent E for both beams and planks can be used directly in traditional beam deflection formulas. The True E values (i.e., shear-free) are for both beams and planks. Using True E, deflection is calculated as follows for uniformly loaded simple span beams:

$$\Delta = [5WL^4/(32Ebh^3)] + [12WL^2/(5Ebh)]$$
 where: Δ = deflection in inches (mm)
 W = uniform load in pounds/inches. (N/mm)
 L = span in inches (mm)
 E = modulus of elasticity in psi (MPa)
 b = width of beam in inches (mm)
 h = depth of beam in inches (mm)
- The bending values in these tables are based on a referenced depth of 12" (305 mm). For other depths, the bending values shall be adjusted by a size factor adjustment of (12/d)^{0.162}, where d is measured in inches with a minimum depth of 2" (51 mm).
- When structural members qualify as repetitive members in accordance with the applicable code, a 4% increase is permitted.
- Design value shall be multiplied by (3.58/L)^{0.125} for length effect factors, with L measured in feet. Value limited to members 16" (406 mm) deep and less.
- Compression value for Y-L plane only.

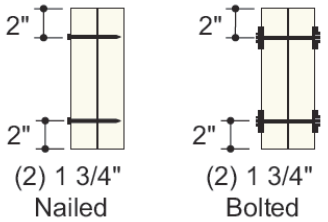
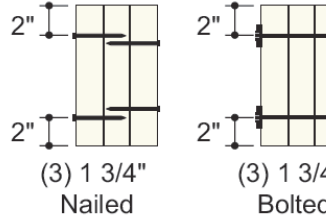
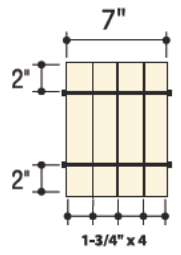
Equivalent Specific Gravities & Minimum Fastener Spacing for Design of Mechanical Connections^{1, 2, 3}

Product	Fastener	Fastener Axis Orientation ¹	Load Direction	Equivalent Specific Gravity for Design Purpose	Maximum Spacing
Ultralam™ LVL	Nails	Y axis	Withdrawal	0.46	See footnote 4
		X axis	Withdrawal	0.45	
	Nails & Screws	Y axis	X and L	0.53	
	Screws	Y axis	X axis	0.53	Per applicable code
		Y axis	L axis	0.49	



1. Orientation nomenclature for Ultralam™ LVL.
2. Adjustment of the design stresses for duration of load shall be in accordance with the applicable code or NDS, as applicable.
3. Connection design values are as provided in NDS for sawn lumber having equivalent specific gravities as shown.
4. Spacing, edge distance and end distance of nails installed perpendicular to the glue lines of the LVL are the same as those permitted in the applicable code for sawn lumber. Spacing of nails installed parallel to the glue lines of the LVL must be a minimum of 3" (76 mm) for 8d (0.131" x 2½") (3.3 mm x 63 mm) common nails, 4" (102 mm) for 10d (0.148" x 3") (3.8 mm x 76 mm) and 12d (0.148" x 3¼") (3.8 mm x 83 mm) common nails. The end distances must be a minimum of 2" (51 mm) for 8d (0.131" x 2½") (3.3 mm x 63 mm) common nails, 3" (76 mm) for 10d (0.148" x 3") (3.8 mm x 76 mm) and 12d (0.148" x 3¼") (3.8 mm x 83 mm) common nails. The minimum nail spacing must be 8" (204 mm) for 16d (0.162" x 3½") (4.1 mm x 89 mm) common nails installed parallel to the glue lines of the LVL that is at least 1¼" thick by 5½" wide (44mm by 133 mm), and the minimum end distance must be 3" (76 mm). Minimum edge distance must be sufficient to prevent splitting of the LVL. In addition, maximum nail penetration into the LVL must be limited as to prevent splitting.

Connection Requirements for Multiple Member Side-Loaded Beams

Assembly A (2-ply Beam)	Assembly B (3-ply Beam)	Assembly C (4-ply Beam)
 <p>(2) 1 3/4" Nailed</p> <p>(2) 1 3/4" Bolted</p>	 <p>(3) 1 3/4" Nailed</p> <p>(3) 1 3/4" Bolted</p>	 <p>7"</p> <p>2"</p> <p>2"</p> <p>1-3/4" x 4</p>

Connection Requirements for Multiple Member Side-Loaded Beams^{1, 2, 3, 4, 5, 6}

Maximum Uniformly Distributed Load (plf) (kgm) that can be Applied to Either Side of the Beam					
Assembly Detail (See Figure Above)	2 Rows of 16d (0.162" x 3 1/2") (4.1 mm x 89 mm) Nails at 12" o.c. (305 mm)	3 Rows of 16d (0.162" x 3 1/2") (4.1 mm x 89 mm) Nails at 12" o.c. (305 mm)	2 Rows of 12d (0.148" x 3 1/4") (3.3 mm x 83 mm) Nails at 12" o.c. (305 mm)	3 Rows of 12d (0.148" x 3 1/4") (3.3 mm x 83 mm) Nails at 12" o.c. (305 mm)	2 Rows of 1/2" (13 mm) Diameter Bolts at 12" o.c. ^{7, 8} (305 mm)
A	620 (923)	935 (1391)	520 (774)	785 (1168)	1480 (2203)
B ⁹	465 (692)	700 (1042)	390 (580)	585 (871)	1110 (1652)
C	-	-	-	-	985 (1466)

For SI: 1 plf = 1.488 kg/m

1. Multiply the appropriate table value by:
 - a. 1.5 for nails or bolts spaced at 8" o.c. (203 mm) per row.
 - b. 2 for nails or bolts spaced at 6" o.c. (152 mm) per row.
 - c. 3 for nails or bolts spaced at 4" o.c. (102 mm) per row.
 - d. 0.5 for bolts spaced at 24" o.c. (610 mm) per row.
2. Determine the appropriate beam size required to support the load before determining the connection requirements.
3. Screws can be used in place of bolts, provided additional fasteners are used such that the sum of the screw capacities is equal to or greater than that of the 1/2"-diameter bolts (13 mm). Refer to the screw manufacturer's literature.
4. Tabulated values assume adequate end distance, edge distance and spacing per Chapter 12 of the 2015 edition of NDS.
5. Tabulated values are for normal load duration. Adjustment of the design stresses for duration of load shall be in accordance with the building code or NDS, as applicable.
6. For beams greater than 4 plies wide, consult a registered design professional for the attachment requirements.
7. A standard cut steel washer of minimum 0.118" thickness (3 mm), with a minimum outside dimension of 1 3/8" (35 mm), is required on each side of the beam between the wood and bolt head and nut.
8. Bolted connections assume full diameter bolts with bending yield strength (F_{yb}) of 45,000 psi (310 Mpa).
9. Nailing is required from both sides for 3-ply beams.

Design Assumptions for Ultralam™ 2.0E/2900Fb Joist & Rafter Tables

SUPPORT REQUIREMENTS

Joists and rafters must have adequate support. Ridge beams must be installed at roof peaks with rafters bearing directly on the ridge beam or supported by hangers or framing anchors. Ceiling joists are not required when properly designed ridge beams are used. A ridge board may be substituted for a ridge beam when the roof slope equals or exceeds 3 in 12, except that ridge beams are required for cathedral ceilings. Ridge boards must be at least 1" nominal in thickness and not less than the depth of the cut end of the rafter. Rafters must be placed directly opposite each other, and ceiling joists must be installed parallel to the rafters to provide a continuous tie between exterior walls.

SPANS

The spans provided in these tables were determined on the same basis as those given in the code-recognized *Span Tables for Joists and Rafters* and *Wood Structural Design Data*, both published by AF&PA. Maximum spans were computed using Allowable Stress Design (ASD) and standard engineering design formulas for simple span beams with uniformly distributed gravity loads. The calculated spans assume fully supported members, properly sheathed and nailed on the top edge of the joist or rafter. They do not, however, include composite action of adhesive and sheathing. Listed spans also do not include checks for concentrated or partition loads that may be required by building codes for specific occupancy or use categories. Uplift loads caused by wind also have not been considered. Spans in the tables are given in feet and inches and are the maximum allowable horizontal span of the member from inside to inside of bearings. For sloping rafters, the span is also measured along the horizontal projection.

REFERENCE DESIGN VALUES

The reference design values used to determine the spans in the accompanying tables are as published in Technical Evaluation Report TER No. 1203-02: *MLT Ultralam™ Laminated Veneer Lumber (LVL)*. Reference design values are based on normal load duration and dry service conditions.

ADJUSTMENT FACTORS

Reference design values must be multiplied by all applicable adjustment factors to determine adjusted design values. Adjusted design values are then used to calculate the maximum allowable span for a specified load condition. The adjustment factors used to develop the accompanying span tables are described below. For more complete information on adjustment factors, refer to TER No. 1203-02 and *NDS®*, *National Design Specification® for Wood Construction*.

REPETITIVE MEMBER FACTOR, C_r – Bending design values, F_b , for the Ultralam™ product listed in these tables are multiplied by the repetitive member factor, $C_r = 1.04$, when such members are in contact or spaced not more than 24" on-center, are not less than three in number, and are joined by floor, roof or other load-distributing elements adequate to support the design load.

LOAD DURATION FACTOR, C_D – Wood has the ability to carry substantially greater loads for short durations than for long durations. Reference design values apply to the normal ten-year load duration. With the exception of modulus of elasticity (E and E_{min}), compression perpendicular-to-grain (F_{cPerp}) and shear modulus of elasticity (G) reference design values must be multiplied by the appropriate load duration factor, C_D . Floor joist and ceiling joist tables are based on the normal load duration, which implies $C_D = 1.0$. For rafters, the load duration factor, C_D , is typically either 1.15 for snow loads (two months duration) or 1.25 for construction loads (seven-day duration). All rafter tables are labeled to indicate the load duration factor used.

CALCULATIONS

The spans provided in these tables are limited to the minimum value calculated for the following design parameters using ASD:

- BENDING (FLEXURE)
- DEFLECTION (BASED ON LIVE LOAD)
- COMPRESSION PERPENDICULAR-TO-GRAIN
- SHEAR PARALLEL-TO-GRAIN (HORIZONTAL SHEAR)

BENDING

Bending design values assume a fully supported member, with structural sheathing nailed on the top edge of the joist or rafter. The repetitive member factor, C_r , of 1.04 was included due to the assumption of the installation of at least three joists or rafters spaced not more than 24" on-center. The load duration factor, C_D , has also been applied as appropriate.

DEFLECTION

Deflection may be the controlling factor in determining the member size required when appearance or rigidity is important. Control of floor vibration is another important reason to limit deflection. Deflection limits are expressed as a fraction of the span length in inches (I), and consider only live load in accordance with established engineering practice for the design of joists and rafters. The live load deflection ratio used to develop each table is listed in the caption for each table.

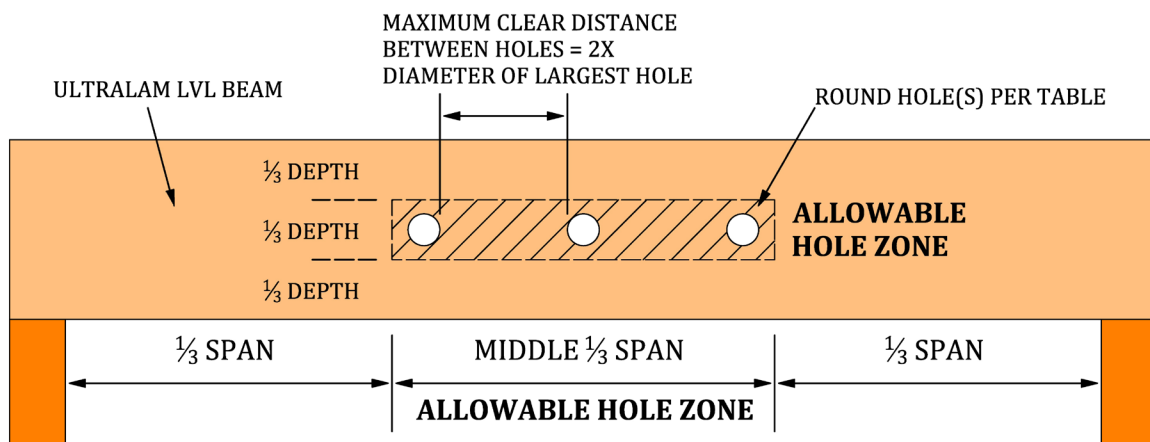
COMPRESSION PERPENDICULAR-TO-GRAIN

The compression perpendicular-to-grain check used to develop these span tables assumes a 2.0" bearing length to account for the end of the joist or rafter bearing on a 2x4 wall with a 1½" rim board applied along the outside edge of the wall. An additional check is required for shorter bearing lengths, such as for 1.5" ledgers.

SHEAR PARALLEL-TO-GRAIN (HORIZONTAL SHEAR)

All uniformly distributed loads within a distance from the inside face of each support equal to the depth of the member have been ignored for determining the maximum allowable span based on horizontal shear.

Allowable Horizontal Holes in Ultralam LVL Beam(s)



NOTES:

1. Hole(s) must be located completely in the allowable hole zone.
2. No rectangular holes are allowed.
3. No more than three (3) holes allowed per span.
4. Table is valid for single span, uniformly loaded beams only. Table is not valid for cantilever sections.
5. Hole location, clearance and effect of beam deflection should be considered to avoid problems with piping.
6. All connections of beams to supports shall be designed by others.
7. Notching of beam not permitted.

Allowable Hole Sizes

Beam Depth	Max Round Hole Diameter
7 ¼" - 9 ¼"	1 ½"
9 ½" - 16"	2"
Deeper than 16"	3"

General Notes for the following 1 ¾" Ultralam™ 2.0E/2900Fb Load Tables

1. All uniform loads given in the tables are in pounds per lineal foot (plf).
2. Floor Systems are designed using a Load Duration Factor (C_D) = 1.0.
3. Roof Systems are designed using C_D = 1.15 or 1.25.
4. All loads are adjusted for the weight of the beam.
5. The top line (LL) of each table indicates the allowable load-carrying capacity using the live load deflection limit.
6. The middle line (TL) of each table indicates the allowable load carrying capacity using the total load deflection limit of the member. Beam self weight is accounted for in the calculations.
7. The bottom line (Brg) of each table indicates the required bearing length at each end of the beam in inches when loaded to the maximum loads allowed and assumes that the compression strength of the bearing material is greater than or equal to the compression perpendicular-to-grain design value of the Ultralam™ beam. Shorter bearing lengths may be required with lighter loads, and longer bearing lengths may be required where the bearing strength of the supporting material is less than the bearing strength of the Ultralam™ beam. Calculations are based on a design span measured from centerline of support to centerline of support. If different bearing lengths are required, design span should be evaluated accordingly.
8. For live load deflection factors of $l/180$ and $l/360$, multiply the maximum live load value (LL) by 1.333 and 0.667, respectively. The result shall not exceed the maximum allowable total load (TL).
9. Design span is assumed to be the clear opening plus ½ the tabulated required bearing length at each end.
10. These tables are for gravity loads only. Consult a professional engineer for wind and seismic load analysis and design.
11. All tables are based on uniform load conditions. Any concentrated load applications must be analyzed separately or converted to an equivalent uniform load.
12. The compression edge of the header or beam must be laterally supported at intervals of 24" or less. In addition, lateral support must be provided at bearing points.
13. For 3 ½" thick Ultralam, use 1 ¾" 2 ply table.
14. Multiply 1.75" load values by 0.857 when using 1.5" thick Ultralam. Do not adjust bearing length.

Ultralam™ LVL 2.0E/2900Fb – Floor Joist Span Tables

30 psf live load, 10 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	17-8	16-0	15-1	14-0	16-0	14-7	13-8	12-9	14-11	13-6	12-9	11-10
1 3/4 x 9 1/2		23-1	21-0	19-9	18-4	21-0	19-1	17-11	16-8	19-6	17-9	16-8	15-6
1 3/4 x 11 7/8		28-11	26-3	24-8	22-11	26-3	23-10	22-5	20-10	24-4	22-2	20-10	19-4
1 3/4 x 14		34-1	30-11	29-1	27-0	30-11	28-1	26-6	24-7	28-9	26-1	24-7	22-10
1 3/4 x 16		38-11	35-4	33-3	30-11	35-4	32-2	30-3	28-1	32-10	29-10	28-1	26-1
1 3/4 x 18		43-10	39-9	37-5	34-9	39-9	36-2	34-0	31-7	36-11	33-7	31-7	29-4
1 3/4 x 24		58-5	53-1	49-11	46-4	53-1	48-2	45-4	42-1	49-3	44-9	42-1	39-1
40 psf live load, 10 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	16-0	14-7	13-8	12-9	14-7	13-3	12-5	11-7	13-6	12-3	11-7	10-9
1 3/4 x 9 1/2		21-0	19-1	17-11	16-8	19-1	17-4	16-4	15-2	17-9	16-1	15-2	14-1
1 3/4 x 11 7/8		26-3	23-10	22-5	20-10	23-10	21-8	20-5	18-11	22-2	20-1	18-11	17-7
1 3/4 x 14		30-11	28-1	26-6	24-7	28-1	25-7	24-0	22-4	26-1	23-9	22-4	20-9
1 3/4 x 16		35-4	32-2	30-3	28-1	32-2	29-2	27-6	25-6	29-10	27-1	25-6	23-8
1 3/4 x 18		39-9	36-2	34-0	31-7	36-2	32-10	30-11	28-8	33-7	30-6	28-8	26-8
1 3/4 x 24		53-1	48-2	45-4	42-1	48-2	43-10	41-3	38-3	44-9	40-8	38-3	35-6
30 psf live load, 15 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	17-8	16-0	15-1	14-0	16-0	14-7	13-8	12-9	14-11	13-6	12-9	11-10
1 3/4 x 9 1/2		23-1	21-0	19-9	18-4	21-0	19-1	17-11	16-8	19-6	17-9	16-8	15-6
1 3/4 x 11 7/8		28-11	26-3	24-8	22-11	26-3	23-10	22-5	20-10	24-4	22-2	20-10	19-4
1 3/4 x 14		34-1	30-11	29-1	27-0	30-11	28-1	26-6	24-7	28-9	26-1	24-7	22-10
1 3/4 x 16		38-11	35-4	33-3	30-11	35-4	32-2	30-3	28-1	32-10	29-10	28-1	26-1
1 3/4 x 18		43-10	39-9	37-5	34-9	39-9	36-2	34-0	31-7	36-11	33-7	31-7	29-4
1 3/4 x 24		58-5	53-1	49-11	46-4	53-1	48-2	45-4	42-1	49-3	44-9	42-1	39-1
40 psf live load, 15 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	16-0	14-7	13-8	12-9	14-7	13-3	12-5	11-7	13-6	12-3	11-7	10-9
1 3/4 x 9 1/2		21-0	19-1	17-11	16-8	19-1	17-4	16-4	15-2	17-9	16-1	15-2	14-1
1 3/4 x 11 7/8		26-3	23-10	22-5	20-10	23-10	21-8	20-5	18-11	22-2	20-1	18-11	17-7
1 3/4 x 14		30-11	28-1	26-6	24-7	28-1	25-7	24-0	22-4	26-1	23-9	22-4	20-9
1 3/4 x 16		35-4	32-2	30-3	28-1	32-2	29-2	27-6	25-6	29-10	27-1	25-6	23-8
1 3/4 x 18		39-9	36-2	34-0	31-7	36-2	32-10	30-11	28-8	33-7	30-6	28-8	26-8
1 3/4 x 24		53-1	48-2	45-4	42-1	48-2	43-10	41-3	38-3	44-9	40-8	38-3	35-6
50 psf live load, 10 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	14-11	13-6	12-9	11-10	13-6	12-3	11-7	10-9	12-7	11-5	10-9	10-0
1 3/4 x 9 1/2		19-6	17-9	16-8	15-6	17-9	16-1	15-2	14-1	16-5	14-11	14-1	13-1
1 3/4 x 11 7/8		24-4	22-2	20-10	19-4	22-2	20-1	18-11	17-7	20-7	18-8	17-7	16-4
1 3/4 x 14		28-9	26-1	24-7	22-10	26-1	23-9	22-4	20-9	24-3	22-0	20-9	19-3
1 3/4 x 16		32-10	29-10	28-1	26-1	29-10	27-1	25-6	23-8	27-8	25-2	23-8	22-0
1 3/4 x 18		36-11	33-7	31-7	29-4	33-7	30-6	28-8	26-8	31-2	28-4	26-8	24-9
1 3/4 x 24		49-3	44-9	42-1	39-1	44-9	40-8	38-3	35-6	41-6	37-9	35-6	33-0
50 psf live load, 20 psf dead load		I/360 deflection				I/480 deflection				I/600 deflection			
Size	Grade	12	16	19.2	24	12	16	19.2	24	12	16	19.2	24
1 3/4 x 7 1/4	2.0E	14-11	13-6	12-9	11-10	13-6	12-3	11-7	10-9	12-7	11-5	10-9	10-0
1 3/4 x 9 1/2		19-6	17-9	16-8	15-6	17-9	16-1	15-2	14-1	16-5	14-11	14-1	13-1
1 3/4 x 11 7/8		24-4	22-2	20-10	19-4	22-2	20-1	18-11	17-7	20-7	18-8	17-7	16-4
1 3/4 x 14		28-9	26-1	24-7	22-10	26-1	23-9	22-4	20-9	24-3	22-0	20-9	19-3
1 3/4 x 16		32-10	29-10	28-1	26-1	29-10	27-1	25-6	23-8	27-8	25-2	23-8	22-0
1 3/4 x 18		36-11	33-7	31-7	29-4	33-7	30-6	28-8	26-8	31-2	28-4	26-8	24-9
1 3/4 x 24		49-3	44-9	42-1	39-1	44-9	40-8	38-3	35-6	41-6	37-9	35-6	33-0

Ultralam™ LVL Stud Allowable Superimposed Axial Load with Studs at 16" o.c. Load Table – 2.0E¹⁻¹⁴

Wall Height	Load Deflection	1.5 in x 5.5 in		1.75 in x 5.5 in		1.5 in x 7.25 in		1.75 in x 7.25 in	
		Interior	Exterior	Interior	Exterior	Interior	Exterior	Interior	Exterior
16	Axial Load Per Stud (lbs) (Deflection Ratio)	4256 (l/2546)	3948 (l/509)	4840 (l/2971)	4836 (l/594)	5637 (l/5780)	5637 (l/1156)	6407 (l/6743)	6407 (l/1348)
18		3535 (l/1793)	2909 (l/358)	4216 (l/2092)	3629 (l/418)	5620 (l/4078)	5620 (l/815)	6390 (l/4757)	6390 (l/951)
20		2850 (l/1309)	2126 (l/261)	3424 (l/1528)	2718 (l/305)	5602 (l/2982)	5602 (l/596)	6373 (l/3479)	6373 (l/695)
22		2306 (l/985)	1524 (l/197)	2794 (l/1149)	2017 (l/229)	5290 (l/2246)	4567 (l/449)	6293 (l/2620)	5645 (l/524)
24		1868 (l/759)	1052 (l/151)	2287 (l/886)	1469 (l/177)	4475 (l/1733)	3601 (l/346)	5349 (l/2022)	4524 (l/404)

1. Allowable superimposed axial line load (plf) is permitted to be calculated by dividing the axial load per stud by 1.333 and shall not be extrapolated to wider stud spacings.
2. The table specifies allowable superimposed load. Dead load of the stud, 2 psf exterior gypsum sheathing, and 2 psf interior gypsum sheathing have been considered.
3. Gypsum Sheathing is assumed to be adequately fastened to both the exterior and interior of the stud wall. The fasteners are assumed to be sufficient such that lateral movement of the stud is restrained.
4. Compression perpendicular to grain of the top and bottom plates is assumed to be 425 psi and is adjusted by a Bearing Area Factor (C_b) calculated in accordance with the National Design Specification for Wood Construction (NDS), based on the thickness of the stud.
5. Blocking between the studs is assumed to be provided 8 ft. on center along the length of the stud.
6. Interior studs are designed for combined axial and bending load with an in-plane uniform live load of 5 psf, using a Duration of Load Factor (C_D) of 1.0.
7. Exterior Studs are designed for combined axial and bending load, with an in-plane uniform wind load of 25 psf, using a Duration of Load Factor (C_D) of 1.6.
8. The wind load of 25 psf is determined using ASCE 7 – 10 Components and Cladding (C&C) with a Mean Roof Height of 33 ft, Exposure Category C, Enclosed Building, 125 mph Ultimate Windspeed, Risk Category II, and a Topographic Factor of 1.0.
9. In determining the 25 psf load, a reduction of 0.6 is applied to the calculated C&C load as allowed in the Load Combinations found in ASCE 7 – 10.
10. The Field Zone, or Zone 4, of the wall is assumed when calculating C&C loading.
11. The stated Components and Cladding Wind Load is reduced by a factor of 0.42 for determining deflection.
12. A value of 0.85 is used for the Column Buckling Length Coefficient (K_e).
13. A Repetitive Member Factor (C_r) of 1.04 is used.
14. A Volume Factor (C_v) of $(12/d)^{0.162}$ is used, where d is the depth of the stud.



State-by-state sealed Technical Evaluation Reports (TERs) can be acquired by visiting www.drjcertification.org/stateselect/75 and selecting the state required. These reports can be used for assuring any building code authority that Ultralam meets all the requirements for use throughout the USA.

Software for sizing and structural checks, isDesign™, is provided at no additional charge. CSD iStruct™ software is widely used throughout the industry. (Visit www.csdsoftware.com.)



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